City of San Antonio

TRANSPORTATION AND CAPITAL IMPROVEMENTS

ADDENDUM No. 4

FORMAL REQUEST FOR COMPETITIVE SEALED PROPOSAL (RFCSP)

PROJECT NAME: FIRE STATION # 32 REPLACEMENT, Project No. 20-00015
DATE: March 10, 2015

This addendum is separated into sections for convenience; however, all contractors, subcontractors, material men, and other parties shall be responsible for reading the entire addendum. The failure to list an item or items in all affected sections of this addendum does not relieve any party affected from performing as per instructions, providing that the information is set forth one time any place in this addendum. These documents shall be attached to and become part of the Contract Documents for this project. The contractor shall be required to sign an acknowledgement of the receipt of this addendum and submit with their proposal package.

I. Allowance

The Contractor will install all rough-ins which include junction boxes, conduit raceways, speaker and device back-boxes for the USDD Alerting System. USDD or USDD approved installer will provide and install the cabling and cable termination to the devices. USDD will provide and install the equipment and programming. The complete installation including USDD equipment is intended to be entirely procured by the General Contractor. The Owner will establish an allowance to cover the cost of USDD provided material and labor. The amount for this allowance is $120,000.00. Please utilize the revised "Price Proposal Form" revised 3/10/15. The revised Price Proposal Form includes this allowance.

II. Clarification to Specifications

Please see the attached information regarding joists.

FORM 6 PROPOSAL FORM (Revised 3/10/15)

The estimated construction budget for this contract is $4,100,000.00

I. BASE BID

Provide and install all materials, labor and construction operations necessary to complete the intended work described and shown in the plan-drawings and technical specifications, for Fire Station No. 32. Fire Station No. 32 is a new free standing 14,725sf 1 story structure to be located on an approximate 3 acre site on Charles Katz Drive in San Antonio, Texas:

Total Amount of Base Bid:_____ **$**_____

(Insert Total Amount of Base Bid in Words and Numbers)

II. ALTERNATES

Amount of each Alternates (if applicable) insert amount in numbers:

Additive Alternate #1 - Sports Flooring

Total Amount of Bid for Additive Alternate #1 (Insert Amount in Words and Numbers):

_____$_____

Additive Alternate #2 – Pedestrian Flasher Equipment

Total Amount of Bid for Additive Alternate #2 (Insert Amount in Words and Numbers):

_____$_____

Additive Alternate #3 - Additional Prefinished Welded Wire Fencing

Total Amount of Bid for Additive Alternate #3 (Insert Amount in Words and Numbers):

_____$_____

Additive Alternate #4 - High Performance Coating for Architecturally Exposed Structural Steel

Total Amount of Bid for Additive Alternate #3 (Insert Amount in Words and Numbers):

_____$_____

Deductive Alternate #5 - Building Pad Preparation - 50% Select Fill and 50% Granular Select Fill

Total Amount of Bid for Deductive Alternate #5 (Insert Amount in Words and Numbers):

_____ $_____

Deductive Alternate #6 - Building Pad Preparation - 100% Select Fill

Total Amount of Bid for Deductive Alternate #6 (Insert Amount in Words and Numbers):

_____ $_____

III. UNIT PRICES

Bidders shall submit unit pricing on the 024 Unit Pricing form, and it shall be attached immediately following this sheet. The unit prices bid shall be the 'complete-in-place unit costs' that is necessary and required to complete the unit bid item work described.

The unit prices bid may be used by the City of San Antonio to change the intended scope and/or the final contract amount for this project by applying "additions-to" or "deletions-form" the scope of work, at the sole discretion of the City of San Antonio.

IV. ALLOWANCES (if applicable)

Allowance 1: Utility Impact Fees $90,000.00
Allowance 2: Salvage Cover Equipment $2,500.00
Allowance 3: Additional Equipment or Furnishings $25,000.00
Allowance 4: Hardware Modifications $3,000.00
Allowance 5: For Owner's Convenience $20,000.00
Allowance 6: USDD Fire Station Alerting Equipment $120,000.00

The Owner reserves the right to select alternates in the order of its choice and at its discretion.

BY MY SIGNATURE BELOW, I certify I legally am authorized to bind Respondent to the terms and conditions contained in this submitted RFCSP Proposal. I further certify the information contained in this submittal accurately reflects data regarding my organization/firm, the work to be performed and the estimates of planned/delivered services. By signing this Proposal Cover/Contract Signature Page, I understand and agree, if awarded a contract in response to this RSCSP, Respondent shall be ready, willing and able to comply with all representations made by Respondent in this Submittal and during the RFCSP Solicitation process.

Respondent certifies it fully shall comply with all of Contract Documents, pursuant to this RFCSP solicitation, for the amount(s) shown and details contained in Respondent's accompanying Proposal Form. Respondent confirms all work proposed by this RFCSP, when fully completed, shall be performed and acceptable to the entire satisfaction of City. As the legal representative of Respondent, I certify all prices contained in this proposal carefully have been checked and are submitted as true, correct and final.

As the legally authorized representative of Respondent, I submit this proposal and, by my signature below, acknowledge that I have received and read the entire RFCSP and each of the RFSCP attachments and agree, on behalf Respondent, to be bound by the terms therein. I further acknowledge I have received all Addenda and agree with and Respondent shall be bound by the terms, conditions and requirements of this submitted proposal, all documents listed in the RFCSP Submittal Checklist and Table

of Contents, the enabling City Ordinance and all of the associated documentation that form the entire Contract to which Respondent shall be bound, upon the approval of the San Antonio City Council.

I certify any objections Respondent may have with the General Conditions for City of San Antonio Construction Contracts, labeled as RFCSP **"Exhibit B"** hereto and incorporated herein, have been listed and included in Respondent's written comments under **Tab 8** hereto. I further certify all provisions contained in this submitted Proposal shall remain valid for 120 calendar days following the posted deadline date for submissions and, if Respondent is awarded a contract, throughout the entire term of the awarded contract.

Official Name of Company (legal)	Telephone No.
Signature of Authorized Individual	Typed Name of Authorized Individual
Date	Typed Title of Authorized Individual
Company Address	Fax No.
City, State and Zip Code	E-mail Address

Name of the proposed **Project Manager:** _____

Name of the proposed **Site Superintendent:** _____

BEATY PALMER ARCHITECTS

ADDENDUM

Date: March 10th, 2015
Project No. 1219

ADDENDUM NO. 04

To the drawings and specifications for:
Fire Station 32
4839 Charles Katz Drive,
San Antonio, Texas

Beaty Palmer Architects, Inc.
110 Broadway, Suite 600
San Antonio, Texas 78205

STRUCTURAL DRAWINGS:

Item 4.1 Design Double Pitched Joists for a uniform dead load of 25 PSF and a uniform live load of 20 PSF. See attached revised Structural drawings for details.

Attachments: Revised Drawings 24x36
S1.00 – Notes & Abbreviations
S4.01 – Roof Framing Details

Mike Beaty, AIA
Principal

03.09.15

Beaty Palmer Architects, Inc.
110 Broadway St., Suite 600
San Antonio, Texas 78205

Tel: +1 210 212 8022

GENERAL NOTES

1. DESIGN LIVE LOADS:

   LIVE LOADS:

OFFICE	50 PSF OR 2000 LB CONC. LOAD PLUS 15 PSF PARTITION LIVE LOAD
LOBBIES	100 PSF
CORRIDORS	100 PSF
RESTROOMS	75 PSF
MECHANICAL EQUIPMENT ROOMS	125 PSF
ALL OTHER INDOOR AREAS	100 PSF
TRUCK BAYS	EQUIPMENT WEIGHT
LIGHT STORAGE	125 PSF
ROOFS	20 PSF

   SEE PLANS FOR SPECIAL LIVE LOAD AREAS.

   (DESIGN LIVE LOADS HAVE BEEN REDUCED IN ACCORDANCE WITH APPLICABLE BUILDING CODE FOR COLUMN AND FOUNDATION DESIGN ONLY.)

2. WIND LOAD DESIGN CRITERIA:

ULTIMATE DESIGN WIND SPEED Vult	120 MPH
NOMINAL DESIGN WIND SPEED Vasd	93 MPH
RISK CATEGORY	IV
WIND EXPOSURE	B
INTERNAL PRESSURE COEFFICIENT	±0.18

3. SNOW LOADS:

GROUND SNOW LOAD	5 PSF
SNOW IMPORTANCE FACTOR	1.10

4. SEISMIC DESIGN CRITERIA:

RISK CATEGORY	IV
SEISMIC DESIGN CATEGORY	A
SEISMIC IMPORTANCE FACTOR (Ie)	1.5
SPECTRAL RESPONSE ACCELERATIONS	
SS =	
S1 =	
SPECTRAL RESPONSE ACCELERATION PARAMETERS	
SDS =	
SD1 =	
BASIC SEISMIC FORCE – RESISTING SYSTEM	
ORDINARY REINFORCED MASONRY SHEAR WALLS & STEEL BRACING	
DESIGN BASE SHEAR =	
ANALYSIS PROCEDURE USED = EQUIVALENT LATERAL FORCE	

5. STRUCTURAL DESIGN IS IN ACCORDANCE WITH PROVISIONS OF THE 2012 INTERNATIONAL BUILDING CODE.

6. REFERENCES TO STANDARDS ARE TO EDITIONS INDICATED IN SPECIFICATIONS.

7. PRINCIPAL OPENINGS ARE INDICATED ON THE DRAWINGS. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SLEEVES, BLOCKOUTS, CURBS, AND INSERTS.

SPECIAL INSPECTIONS AND STRUCTURAL OBSERVATIONS

1. THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE FOR THIS PROJECT IS THE ARCHITECT OF RECORD.

2. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON, EMPLOYED BY AN INDEPENDENT AGENCY HIRED BY THE OWNER OR OWNER'S REPRESENTATIVE, WHO SHALL DEMONSTRATE COMPETENCE FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION TO THE SATISFACTION OF THE BUILDING OFFICIAL.

3. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL TESTING AND INSPECTIONS AS WELL AS NOTIFYING THE ENGINEER AND SPECIAL INSPECTORS OF WORK READY FOR OBSERVATION/INSPECTION. THE GENERAL CONTRACTOR MUST PROVIDE ACCESS TO AND MEANS FOR PROPER INSPECTION OF SUCH WORK.

4. SPECIAL INSPECTORS ARE RESPONSIBLE FOR VERIFYING THAT THE DESIGNATED WORK HAS BEEN PERFORMED IN COMPLIANCE WITH THE PROJECT DOCUMENTS AND WITH THE REQUIREMENTS AND STANDARDS OF THE BUILDING CODE. THE INSPECTOR MAY NOT ALTER, MODIFY, ENLARGE, OR WAIVE ANY OF THE REQUIREMENTS OF THE PROJECT DOCUMENTS.

5. SPECIAL INSPECTORS SHALL BRING NONCOMPLIANT ITEMS TO THE IMMEDIATE ATTENTION OF THE GENERAL CONTRACTOR AND THE ARCHITECT/ENGINEER.

6. SPECIAL INSPECTORS SHALL SUBMIT A WEEKLY FIELD REPORT ADDRESSING ALL OUTSTANDING DISCREPANCIES TO THE OWNER, THE BUILDING OFFICIAL, AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE UNTIL ALL CORRECTIONS HAVE BEEN COMPLETED.

7. EACH SPECIAL INSPECTOR IS RESPONSIBLE TO PREPARE, SIGN, AND SUBMIT TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE A REPORT STATING THAT THE CONSTRUCTION WORK REQUIRING SPECIAL INSPECTIONS WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PROJECT DOCUMENTS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THEIR RESPECTIVE CODE. THIS REPORT SHALL BE PREPARED ON A FORM APPROVED BY THE BUILDING OFFICIAL.

8. SPECIAL INSPECTIONS SHALL BE PROVIDED DURING CONSTRUCTION FOR THE FOLLOWING:

NO.	TYPES OF WORK	CODE SECTION	QUALIFICATIONS AND FREQUENCY	APPLICABLE
1	PIER/FOUNDATION	1705.7/1705.8	–	NO
2	FOOTING BEARING	1705.6	–	YES
3	CONCRETE CONSTRUCTION	1705.3	SEE SPECIFICATIONS	YES
4	STRUCTURAL STEEL	1705.2.1/1705.2.2	SEE SPECIFICATIONS	YES
5	INSPECTION OF FABRICATORS	1704.2	SEE SPECIFICATIONS	NO
6	MASONRY CONSTRUCTION	1704.5	–	YES
7	STRUCTURAL WOOD	1705.5	–	NO

9. WHERE SPECIAL INSPECTION REQUIREMENTS DUPLICATE THE REQUIREMENTS OF OTHER SPECIFIED TESTING, THE MORE STRINGENT OF THE TWO IS REQUIRED.

10. STRUCTURAL OBSERVATION, AS DEFINED BY CHAPTER 17 OF THE BUILDING CODE, IS NOT REQUIRED.

11. STRUCTURAL OBSERVATION IS A VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM BY A REGISTERED DESIGN PROFESSIONAL FOR THE SOLE PURPOSE OF DETERMINING IF THE WORK IS PROCEEDING IN GENERAL ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS AND IS NOT INTENDED TO BE A COMPREHENSIVE REVIEW OF THE QUALITY AND/OR QUANTITY OF WORK.

FOUNDATION NOTES

1. DESIGN SOIL PRESSURE:

   TOTAL LOAD = 3000 PSF
   DEAD LOAD = 2000 PSF

2. SEE GEOTECHNICAL REPORT FOR BUILDING PAD PREPARATION REQUIREMENTS.

3. NOTIFY ARCHITECT/ENGINEER 48 HOURS PRIOR TO INSTALLATION OF FOOTINGS.

4. THE FOUNDATION DESIGN IS BASED ON SUBSURFACE INFORMATION AND RECOMMENDATIONS CONTAINED IN THE REPORT PREPARED BY TERRACON CONSULTANTS,INC. DATED OCTOBER 3, 2014. REPORT NO. 90135072R

CONCRETE NOTES

1. CONCRETE SHALL HAVE SAND AND GRAVEL OR CRUSHED STONE AGGREGATES AND SHALL HAVE A COMPRESSIVE STRENGTH OF 4000 PSI IN 28 DAYS.

2. CONCRETE PROTECTION FOR DEFORMED BAR CONCRETE REINFORCEMENT SHALL BE AS INDICATED BELOW. SEE SECTION 7.7, ACI 318, FOR CONDITIONS NOT INDICATED.

CONCRETE CAST AGAINST EARTH	
(UNLESS NOTED OTHERWISE)	3"
SLABS–ON–GRADE	
#5 BARS AND SMALLER	2" TOP 1½" BOTTOM
#6 BARS AND SMALLER	2" TOP 2" BOTTOM
CONCRETE NOT EXPOSED TO EARTH, WEATHER OR DEICING SALTS	
WALLS	¾"
FOOTINGS/MATS	3"
SLABS	¾"
CONCRETE EXPOSED TO EARTH, WEATHER OR DEICING SALTS	
GRADE BEAMS (SIDES FORMED)	3" BOTTOM
#3 STIRRUPS	1⅜" SIDES AND TOP
#4 OR #5 STIRRUPS	1½" SIDES AND TOP
#6 STIRRUPS	2" SIDES AND TOP
BEAMS	
#4 OR #5 STIRRUPS	2" TOP 1½" SIDES AND BOTTOM
SLABS	
#5 BARS AND SMALLER	2" TOP 1½" BOTTOM
PEDESTALS	
#4 OR #5 TIES OR SPIRALS	1½"

4. LOCATE JOINTS NOT INDICATED TO LEAST IMPAIR STRENGTH AND APPEARANCE OF STRUCTURE. LOCATE HORIZONTAL JOINTS IN CONCRETE ONLY WHERE THEY NORMALLY OCCUR OR WHERE INDICATED. LOCATE VERTICAL JOINTS IN MIDDLE THIRD OF SPANS OF SLABS, BEAMS OR GIRDERS,UNLESS A BEAM INTERSECTS A GIRDER AT MIDDLE LOCATION, IN WHICH CASE OFFSET JOINTS IN GIRDERS TWICE WIDTH OF BEAM.

5. FOR SLABS–ON–GRADE REINFORCED WITH DEFORMED BAR CONCRETE REINFORCEMENT 12'–6" IS THE MAXIMUM SPACING FOR CONTROL JOINTS AT 5" SLAB THICKNESS, AND 20'–0" AT 8" SLAB THICKNESS.

6. ROUGHEN SURFACE OF HORIZONTAL OR NEARLY HORIZONTAL CONSTRUCTION JOINTS SO THAT AGGREGATE SHALL BE EXPOSED UNIFORMLY, LEAVING NO LAITANCE, LOOSENED PARTICLES OR DAMAGED CONCRETE.

7. FURNISH DEFORMED BAR CONCRETE REINFORCEMENT, #3 BARS THROUGH # 11 BARS, CONFORMING TO FOLLOWING:

   ASTM A615, GRADE 60

8. FIELD BENT DOWELS SHALL CONFORM TO ASTM A615, GRADE 40.

9. ELECTRICALLY–WELDED WIRE FABRIC OF COLD–DRAWN WIRE (70,000 PSI YIELD) SHALL CONFORM TO ASTM A185, OF SIZE INDICATED ON DRAWINGS.

10. REINFORCEMENT SPECIFICALLY INDICATED ON DRAWINGS TO BE WELDED SHALL CONFORM TO ASTM A706. WELDING SHALL CONFORM TO AWS D1.4.

11. DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL CONFORM TO ACI 315.

12. SPLICE REINFORCING BARS ONLY AS INDICATED ON DRAWINGS EXCEPT LAP SPLICE REINFORCING BARS DESIGNATED AS "CONTINUOUS" OR "CONT." WITH CLASS A LAP SPLICES. LAP SPLICE CONTINUOUS REINFORCING BARS AT SUPPORT FOR BOTTOM BARS AND AT MIDSPAN FOR TOP AND SIDE BARS.

13. HOOK UNSCHEDULED TOP AND SIDE REINFORCING BARS AT DISCONTINUOUS ENDS.

14. PROVIDE CHAMFERS AS DETAILED ON ARCHITECTURAL DRAWINGS. PLACEMENT OF SLEEVES OR OPENINGS THROUGH BEAMS IS NOT PERMITTED UNLESS INDICATED ON STRUCTURAL DRAWINGS OR ACCEPTED IN WRITING BY ARCHITECT. OPENINGS 12" SQUARE OR SMALLER MAY BE PLACED IN WALLS.

1. REMOVAL OF FORMWORK:

   A. EXCEPT AS HEREIN SPECIFIED, REMOVE FORMWORK AND RESHORE IN ACCORDANCE WITH ACI 301 AND RECOMMENDATIONS OF ACI 347 TO ENSURE COMPLETE SAFETY OF FORMWORK AND STRUCTURE.

   B. FORMWORK FOR COLUMNS, WALLS, SIDES OF BEAMS AND OTHER PARTS NOT SUPPORTING WEIGHT OF CONCRETE MAY BE REMOVED AFTER CONCRETE HAS HARDENED SUFFICIENTLY TO RESIST DAMAGE FROM FORMWORK REMOVAL OPERATIONS.

STRUCTURAL STEEL NOTES

1. STRUCTURAL STEEL ROLLED SHAPES SHALL CONFORM TO ASTM A992. PLATES, ANGLES AND CHANNELS SHALL CONFORM TO ASTM A36, UNLESS NOTED OTHERWISE. COORDINATE GALVANIZING AND FINISH REQUIREMENTS WITH ARCHITECTURAL PAINT REQUIREMENTS.

2. ANCHOR RODS SHALL CONFORM TO ASTM F1554–GRADE 36 UNLESS NOTED OTHERWISE ON THE DRAWINGS.

3. PAINT STRUCTURAL STEEL IN ACCORDANCE WITH SPECIFICATIONS, UNLESS NOTED OTHERWISE.

4. STRUCTURAL STEEL NOT RECEIVING FIREPROOFING SHALL BE PAINTED IN ACCORDANCE WITH SPECIFICATIONS, UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR FIREPROOFING REQUIREMENTS.

5. TOUCH UP FIELD WELDS AND CONNECTIONS OF PAINTED STRUCTURAL STEEL WITH SAME PAINT AS USED IN SHOP.

6. UNLESS NOTED OTHERWISE, GALVANIZE STRUCTURAL STEEL MEMBERS AND EMBEDS EXPOSED TO ELEMENTS AND WHERE INDICATED ON DRAWINGS. GALVANIZING SHALL CONFORM TO ASTM A123. SEAL WELD CONNECTIONS PRIOR TO GALVANIZING.

7. TOUCH UP FIELD WELDS ON GALVANIZED ITEMS WITH PAINT CONFORMING TO TT–P–641.

8. STRUCTURAL STEEL PIPE SHALL CONFORM TO ASTM A501 OR A53, TYPES E OR S.

9. STRUCTURAL STEEL TUBE SHALL CONFORM TO ASTM A500, TYPE B.

10. STRUCTURAL STEEL DETAILS AND CONNECTIONS SHALL CONFORM TO THE STANDARDS OF THE AISC.

11. SELECT CONNECTIONS NOT INDICATED ON DRAWINGS FROM TABLE II, PART 4TH AISC MANUAL. TABLE III, PART 4 MAY BE USED IN COMBINATION WITH TABLE II.

12. DESIGN BEAM–TO–BEAM AND BEAM–TO–COLUMN SHEAR CONNECTIONS FOR ONE–HALF OF SHEAR CAPACITY OF WEB, OR REACTION INDICATED, WHICHEVER IS GREATER.

13. SPLICING OF STRUCTURAL STEEL MEMBERS WHERE NOT INDICATED IS PROHIBITED.

14. WELDING OF STRUCTURAL STEEL SHALL CONFORM TO AWS D1.1. USE E70xxELECTRODES FOR FIELD AND SHOP WELDS. USE ONLY LOW–HYDROGEN ELECTRODES ON ASTM A242, A514, A572 AND A588 STEEL.

15. CONNECTION BOLTS FOR STRUCTURAL STEEL MEMBERS SHALL CONFORM TO ASTM A325 OR A490, UNLESS NOTED OTHERWISE.

16. ERECT STEEL BEAMS WITH CAMBER AS INDICATED ON DRAWINGS. ERECT BEAMS WITH NATURAL CAMBER UP.

17. DO NOT ATTACH EXTERIOR WALL ELEMENTS TO STEEL FRAMING UNTIL DECK HAS BEEN ATTACHED TO FRAMEWORK AND STRUCTURAL BRACING IS IN–PLACE (OR ADEQUATE TEMPORARY BRACING HAS BEEN INSTALLED),EXTERIOR WALL ELEMENTS ATTACHING TO STEEL FRAMING SHALL HAVE CONNECTIONS WHICH ALLOW FOR BOTH HORIZONTAL AND VERTICAL ADJUSTMENT TO COMPENSATE FOR MEMBER ROTATION AND DEFLECTION.

STEEL DECK NOTES

1. WHERE INDICATED ON DRAWINGS, STEEL ROOF DECK RD1, SHALL BE 1½" DEEP X 0.0358" DESIGN THICKNESS (20 GAGE), INTERMEDIATE RIB GALVANIZED DECK AND WELDS WITH A DIAPHRAGM CAPACITY OF 450 PLF AND WITH AN UPLIFT CAPACITY PER DIAGRAMS ON SHEET S5.01. DECK SHALL HAVE A MINIMUM SECTION MODULUS WITH REFERENCE TO TOP OF DECK OF AT LEAST 0.234 CUBIC INCHES PER FOOT OF WIDTH, A MINIMUM SECTION MODULUS WITH REFERENCE TO BOTTOM OF DECK OF AT LEAST 0.247 CUBIC INCHES PER FOOT OF WIDTH AND A MOMENT OF INERTIA OF AT LEAST 0.201 INCHES TO THE FOURTH PER FOOT OF WIDTH. DECK SHALL HAVE A MINIMUM YIELD STRENGTH OF 33000 PSI.

2. WHERE INDICATED ON DRAWINGS, TORIS ROOF DECK, SHALL BE 2⅓" DEEP X 0.0474" DESIGN THICKNESS (18 GAGE), GALVANIZED DECK AND WELDS WITH A WITH AN UPLIFT CAPACITY PER DIAGRAMS ON SHEET S5.01. DECK SHALL HAVE A MINIMUM YIELD STRENGTH OF 40000 PSI.

STEEL JOIST NOTES

1. STEEL JOISTS AND BRIDGING SHALL CONFORM TO STANDARDS OF THE STEEL JOIST INSTITUTE.

2. STEEL JOISTS SHALL HAVE DOUBLE ANGLE TOP AND BOTTOM CHORDS.

3. DESIGN STEEL JOISTS AT ROOF FOR UPLIFT NORMAL TO ROOF SURFACE PER DIAGRAM ON SHEET S5.01

4. ATTACH CONCENTRATED LOADS TO JOISTS AT JOIST PANEL POINTS ONLY.

5. WHERE JOIST BOTTOM CHORD EXTENSIONS ARE INDICATED, DO NOT ATTACH TO COLUMNS, BEAMS, OR WALLS UNTIL ALL ROOF DEAD LOAD IS IN PLACE.

6. DESIGN DOUBLE PITCHED JOISTS FOR A UNIFORM DEAD LOAD OF 25 PSF AND A UNIFORM LIVE LOAD OF 20 PSF.

REINFORCED GROUTED CONCRETE MASONRY UNITS (CMU)

1. REINFORCED GROUTED MASONRY IS DESIGNED FOR AN f'm OF 1500 PSI USING MEDIUM WEIGHT MASONRY BLOCKS OF 8" NOMINAL THICKNESS WITH 2 CELLS PER 16" LENGTH OF BLOCK. UNLESS NOTED OTHERWISE, USE RUNNING BOND CONSTRUCTION.

2. VERTICALLY REINFORCE GROUTED MASONRY WITH #4 AT 24" MINIMUM IN GROUTED CELLS UNLESS NOTED OTHERWISE ON DRAWINGS.

3. HORIZONTALLY REINFORCE GROUTED MASONRY WITH A MINIMUM OF TWO 9 GAUGE OR 3/16" DIAMETER WIRES IN A TRUSS OR LADDER TYPE CONFIGURATION AT 16" IN BED JOINTS. SEE DRAWINGS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS.

4. LOCATE BOND BEAMS VERTICALLY AT 48" MAXIMUM, AT ALL SILLS AND TOP OF ALL WALLS. REINFORCE BOND BEAMS WITH 2–#4 CONTINUOUS BARS, UNLESS NOTED OTHERWISE IN THE DRAWINGS. OMIT HORIZONTAL BED JOINT REINFORCEMENT WHERE BOND BEAMS OCCUR.

5. FURNISH DEFORMED BAR REINFORCING, #3 BARS THROUGH #11 BARS, CONFORMING TO ASTM A615, GRADE 60.

6. LAP SPLICE REINFORCEMENT 48 BAR DIAMETERS BUT NOT LESS THAN 18".

7. UNLESS NOTED OTHERWISE, DOWEL ALL REINFORCED MASONRY WALLS INTO SUPPORTS WITH MATCHING SIZE AND SPACING OF WALL VERTICAL REINFORCEMENT.

8. PROVIDE VERTICAL CONTROL JOINTS IN REINFORCED MASONRY WALLS WITH SPACING NOT TO EXCEED 20 FEET. PROVIDE #4 x 4'–0" LONG SMOOTH DOWELS AT 16" ACROSS THE JOINT. PREVENT BOND BETWEEN THE BAR AND GROUT WITH GREASE OR PLASTIC SLEEVE. CAP ALL DOWELS TO ALLOW FOR 1" OF MOVEMENT.

ABBREVIATIONS

&	AND
∠	ANGLE
@	AT
#	NUMBER OR POUND
AB	ANCHOR BOLT
ACI	AMERICAN CONCRETE INSTITUTE
ADDL	ADDITIONAL
ADJ	ADJACENT
AISC	AMERICAN INSTITUTE OF STEEL CONST
ALT	ALTERNATE
APPD	APPROVED
APPROX.	APPROXIMATE
ARCH	ARCHITECT,ARCHITECTURAL
ASTM	AMERICAN SOCIETY FOR TESTING MATERIAL
BB	BOND BEAM
BF	BOTTOM FACE
BL	BUILDING LINE
BLDG	BUILDING
BLK	BLOCK
BLKG	BLOCKING
BM	BEAM
BOT	BOTTOM
B/	BOTTOM OF
BRG	BEARING
BT	BENT
BTR	BATTER
BTWN	BETWEEN
CG	CENTER OF GRAVITY
CIP	CAST IN PLACE
CIR	CIRCLE, CIRCULAR
CJ	CONSTRUCTION OR CONTROL JOINT
CL	CENTERLINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONNX	CONNECTION
CONSTR	CONSTRUCTION
CONT	CONTINUOUS
CONTR	CONTRACTOR
COORD	COORDINATE
COR	CORNER
CTR	CENTER
CU	CUBIC
D	DEPTH
DBA	DEFORMED BAR ANCHOR
DBL	DOUBLE
DEG, °	DEGREE
DES	DESIGN
DTL	DETAIL
DIA, ø	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DIR	DIRECTION
DL	DEAD LOAD
DN	DOWN
DP	DEEP
DWG	DRAWING
DWL	DOWEL
E	EAST
EA	EACH
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR
EMBED	EMBEDMENT
ENCL	ENCLOSURE
ENGR	ENGINEER
EOS	EDGE OF SLAB
EQ	EQUAL
EQUIP	EQUIPMENT
EQUIV	EQUIVALENT
ES	EACH SIDE
EST	ESTIMATE
EW	EACH WAY
EXC	EXCAVATE, EXCAVATION
EXIST	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
FAB	FABRICATE, FABRICATOR
FD	FLOOR DRAIN
FDN	FOUNDATION
FIN	FINISH, FINISHED
FLG	FLANGE
FLR	FLOOR
FR	FRAME
FT	FEET, FOOT
FTG	FOOTING
GA	GAUGE
GALV	GALVANIZED
GEN	GENERAL
GR	GRADE
GRD	GROUND
H	HORIZONTAL
HGR	HANGER
HGT	HEIGHT
HK	HOOK
HP	HIGH POINT
HS	HEADED STUDS
ID	INSIDE DIAMETER
IF	INSIDE FACE
IN	INCH
INT	INTERIOR
ISOL	ISOLATION
JST	JOIST
JT	JOINT
K	KIPS
KO	KNOCK OUT
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
L	LENGTH
LBS	POUNDS
LIN	LINEAR
LL	LIVE LOAD
LLBB	LONG LEG BACK TO BACK
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LP	LOW POINT
LTWT	LIGHT WEIGHT
LW	LIGHT WEIGHT CONCRETE
MFR	MANUFACTURER
MAS	MASONRY
MAX	MAXIMUM
MB	MACHINE BOLT
MECH	MECHANICAL
MEZZ	MEZZANINE
MID	MIDDLE
MIN	MINIMUM
MISC	MISCELLANEOUS
MK	MARK
ML	MATCH LINE
MTL	METAL
N	NORTH
NIC	NOT IN CONTRACT
NO, #	NUMBER
NOM	NOMINAL
NTS	NOT TO SCALE
NW	NORMAL WEIGHT CONCRETE
OA	OVER–ALL
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OH	OPPOSITE HAND
OPNG	OPENING
OPP	OPPOSITE
PART	PARTITION
PC	PRECAST
PCF	POUNDS PER CUBIC FOOT
PCY	POUNDS PER CUBIC YARD
PL, ▱	PLATE
PLUMB	PLUMBING
PLYWD	PLYWOOD
PNL	PANEL
PREFAB	PREFABRICATED
PRELIM	PRELIMINARY
PROJ	PROJECT, PROJECTION
PS	PRESTRESS
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POST–TENSION, POINT
PVMT	PAVEMENT
QTR	QUARTER
R	RADIUS, RISER, REMAINDER
RB	RAISER BAR
RD	ROOF DRAIN
REF	REFERENCE
REINF	REINFORCING, REINFORCE, REINFORCEMENT
REQD	REQUIRED
REV	REVISION
RF	ROOF
RND	ROUND
RO	ROUGH OPENING
S	SOUTH
SB	SPACER BAR
SC	SHEAR CONNECTOR
SCHED	SCHEDULE
SDL	SUPERIMPOSED DEAD LOAD
SECT	SECTION
SGL	SINGLE
SHT	SHEET
SIM	SIMILAR
SL	SLAB
SP	SPIRAL
SPA	SPACES, SPACING
SPEC	SPECIFICATIONS
SQ	SQUARE
SS	STAINLESS STEEL
STA	STATION
STD	STANDARD
ST	STIRRUP
STIFF	STIFFENER
STL	STEEL
STR	STRAIGHT
STRUCT	STRUCTURAL
SUPPT	SUPPORT
SYMM	SYMMETRICAL
T	TREAD, TOP
T/	TOP OF
T & B	TOP AND BOTTOM
TANG	TANGENT
TEMP	TEMPERATURE, TEMPORARY
THK	THICK, THICKNESS
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
ULT	ULTIMATE
V, VERT	VERTICAL
VAR	VARIES, VARIABLE
VOL	VOLUME
W	WEST, WIDTH
W/	WITH
W/O	WITHOUT
WD	WOOD
WL	WIND LOAD
WP	WORK POINT
WT	WEIGHT
WWR	WELDED WIRE REINFORCEMENT
YD	YARD

PROJECT

Fire Station No. 32

4389 Charles Katz Drive.
San Antonio, Texas

REVISIONS

1	ADDENDUM 04	03/09/15

CONSULTANT

ENGINEER

campbell & associates
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188200

STEPHEN J. CAMPBELL
36249
REGISTERED PROFESSIONAL ENGINEER
03-09-2014

ARCHITECT

DRAWN BY JOW
REVIEWED BY BBY
PROJECT NUMBER 1219
DATE 02.10.15
SHEET NUMBER S1.00

100% CONSTRUCTION DOCUMENTS

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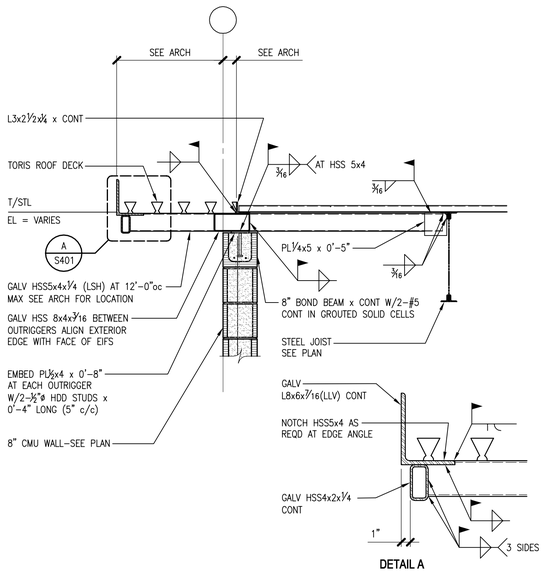
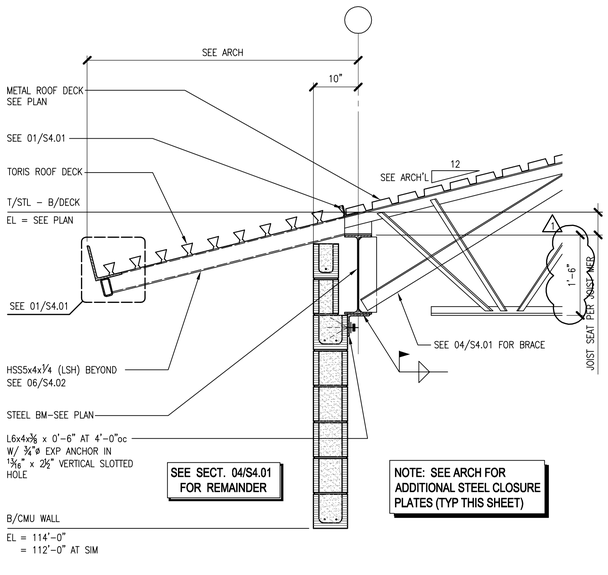
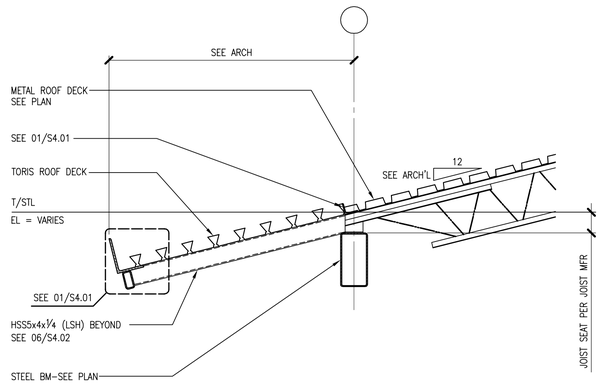


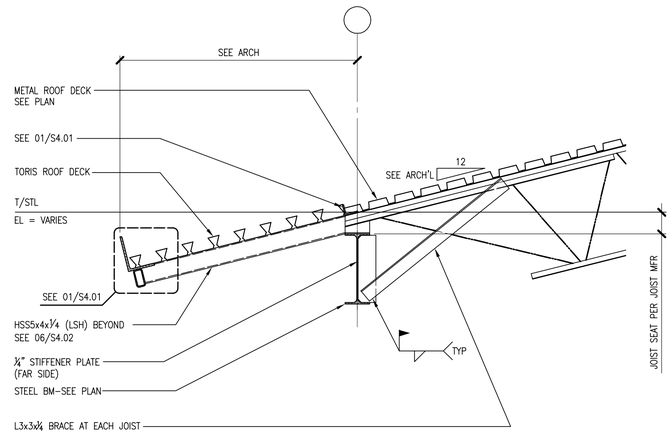
DETAIL A



SEE SECT. 04/S4.01 FOR REMAINDER

NOTE: SEE ARCH FOR ADDITIONAL STEEL CLOSURE PLATES (TYP THIS SHEET)





01 SECTION
SCALE: 3/4" = 1'–0"

02 SECTION
SCALE: 3/4" = 1'–0"

03 SECTION
SCALE: 3/4" = 1'–0"

04 SECTION
SCALE: 3/4" = 1'–0"

05 SECTION
SCALE: 3/4" = 1'–0"

06 SECTION
SCALE: 3/4" = 1'–0"

07 SECTION
SCALE: 3/4" = 1'–0"

PROJECT

Fire Station No. 32

4389 Charles Katz Drive.
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REVISIONS

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DRAWN BY: JOW
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City of San Antonio

TRANSPORTATION AND CAPITAL IMPROVEMENTS

RECEIPT OF ADDENDUM NUMBER(S) **4** IS HEREBY ACKNOWLEDGED FOR THE FIRE STATION # 32 REPLACEMENT, PROJECT NO: 20-00015

FOR WHICH PROPOSALS WILL BE OPENED AND READ ALOUD ON MARCH 17, 2015

*****THIS ACKNOWLEDGEMENT MUST BE SIGNED AND RETURNED WITH THE PROPOSAL PACKAGE.*****

Company Name: _____

Address: _____

City/State/Zip Code: _____

Date: _____

_____

Signature

_____

Print Name/Title